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## Handling & Storage

Vinidex PE pipes are available in a range of sizes ranging from 16mm to 1000mm in configurations complying with AS/NZS4130. Pipes may be supplied to customer requirements in either small diameter pipe in coil lengths up to 9500m, or in straight lengths up to 25m. Vinidex PE pipes are robust, flexible, and offer the installer many cost saving advantages. Whilst they are resistant to site damage, normal care and good housekeeping practices are necessary to ensure trouble free operations.

### Handling

Handling of Vinidex PE pipes is made easier due to the light weights of both coiled and straight length pipe. Care must be exercised however, to avoid damage to the pipe walls, pre-assembled end fittings, or sub assemblies.

Safety aspects need to be addressed, as the nature of PE pipes is such that in cold and wet weather the pipes become slippery and difficult to handle. In these circumstances, additional care should be exercised when handling coils or bundles of pipe. In hot weather, especially with black pipes, the pipe surface may reach 70°C, when the ambient temperatures reach 40°C. Handling PE pipes at these temperatures requires gloves, or other protection, to prevent the possibility of skin burns.

Fabric slings are recommended for lifting and handling PE pipe in order to prevent damage.

Where wire ropes or chains are used, then all of the contact points between the slings and the pipe must be protected by suitable padding. Where pipes are in coils, the slings must be placed evenly around the entire coil. Similarly, where coils or straight lengths are lifted by fork lift the contact points must be protected. When lifting coils, the lifting must be performed on the entire coil, and the fork lift tynes not inserted into the coil winding. When lifting packs of pipes, the tynes must be placed under the entire pack, and the tynes not pushed into the pack. Pipes must not be lifted by placing metal hooks into the ends of straight lengths.

In conditions approaching freezing, the impact resistance of PE reduces, and care must be exercised to prevent damage during handling.

Pipe lengths greater than 6 metres should be lifted using a spreader bar, and wide band slings. PE pipes will flex during lifting, and care needs to be exercised to prevent damage to pipes or end fittings arising from contact with the ground. Care needs to be taken to centre the pipe in the slings.

A reduction in the pipe wall thickness of up to 10% may be tolerated. However, sections with sharp notches should be rejected, or the damaged area buffed out to remove the sharp edges.

### Transport

PE pipes stacked for transport must be evenly supported in order to prevent distortion. All bearing surfaces must be free from contact with sharp objects. Any projecting sections such as stub flanges must be supported to prevent damage.

For straight lengths of pipe, suitable support beneath the pipes is provided by beams of minimum width 75 mm, spaced horizontally at 1.5 m centres. For rectangular stacks, additional vertical supports at 3 metre spacing should be used. For pyramid stacks, the bottom pipe layers also need to be chocked to prevent stack collapse.

For large diameter pipes (DN 630 and above) it may be necessary to tom, or internally support the ends of the pipe in order to prevent distortion.

Where end treatments such as flanges are applied in the factory, these treatments must be protected from damage.

Where coils are stacked vertically the stacks may need to be restrained in order to prevent the bottom section of the coil being flattened or distorted.



## Site Preparation

### Trench Preparation

All other services must be located (such as telephone conduits, gas, water mains, sewers, electrical conduits, and cable TV conduits) in the area of the PE pipeline before any work commences. This may require some localised excavation, and all safety requirements must be observed.

When pipes are installed on the natural surface, the pipeline route must be clear of obstructions and where required, sufficient space must be allowed for expansion/contraction movement.

PE pipes may be joined outside the trench, allowing narrower trenches and consequent reduced excavation cost.

PE pipes have a density less than that of water, and may float if water is present in the trench, and the pipes are not restrained. Trench excavations need to be kept free of water, and if necessary, dewatering equipment installed.

### Trench Widths

Table 5.2 lists recommended trench widths. These values are consistent with the principles that trench width should be as narrow as possible in order to minimise external loads and installation costs, whilst also affording sufficient space to provide the specified compaction.

The actual trench width adopted will be influenced by the soil conditions, the jointing systems, and whether joints are made in the trench.

**Table 5.2 Recommended Trench Widths**

Pipe Diameter ( mm )	Minimum Trench Width (mm)
16 to 63	150
75 to 110	250
125 to 315	500
355 to 500	700
630 to 710	910
800 to 1000	1200

**Table 5.3 Minimum Cover**

Installation Condition	Cover over pipe crown (mm)	
Open Country	300	
Traffic Loading	No pavement	450
	Sealed pavement	600
	Unsealed pavement	750
Construction equipment	Construction equipment	750
	Embankment	750

Poor soil conditions may necessitate a wider trench to accommodate support structures or dewatering equipment, and the ready removal of this equipment after the pipes have been laid. Where such supports are used, they must be removed with care, in order to prevent disturbance of pipe, bedding or trench walls.

Pressure pipes, especially in rural areas, may be installed in narrow trenches with sufficient space to allow the backfill of the trench. No additional compaction may be necessary, and the natural soil consolidation allowed to occur with time.

Where PE pipes are installed with other services in common trench situations, the trench width may be specified by Local Authority regulations in order to permit later maintenance activities.

### Trench Depths

Where the PE pipe grade line is not specified, the cover over the top of the PE pipes needs to be set so that adequate protection from external loads, third party damage, and construction traffic is provided.

Where possible, pipes should be installed under minimum depth conditions and, as a guide, the values listed in Table 5.3 above should be adopted.

Trench walls in poor soil conditions may need to be excavated in steps, or be battered, to prevent collapse of the trench wall materials.

For embankment installations, a sub trench may be excavated once the embankment has been partly built up, in order to help protect the PE pipes from construction vehicles, and also lessen the external loads acting on the pipe.

## Bedding Material

The excavated trench floors must be trimmed even, and be free from all rocks, and hard objects.

In poor soil conditions, an additional layer of imported bedding material may need to be introduced, and a geofabric restraint of bedding/backfill material may be required.

The bedding materials used in both trenches and embankments shall follow the guidelines of AS2033, and should be one of the following:

1. Sand or soil, free from rocks greater than 15 mm, and any hard clay lumps greater than 75 mm in size.
2. Crushed rock, gravel, or graded materials of even grading with a maximum size of 15 mm.
3. Excavated material free from rocks or vegetable matter.
4. Clay lumps which can be reduced to less than 75 mm in size.

Excavated materials in accordance with 3. and 4. above are often used for pressure pipelines and in rural areas.

However, in areas of high loading, such as under roads, imported materials may need to be used.

In the majority of PE pipe applications, a minimum of 75 mm of bedding material is used in both trenches and embankments in soil excavations. For excavations in rock, 150 mm bedding depth may be required.

Where fittings or mechanical joints are used, the bedding material may need to be excavated to prevent point loading. All pegs and markers used in aligning and levelling the pipes must be removed from the trench floor prior to bedding materials being placed.

## Side Support & Overlay

PE pipes act as flexible pipes to resist external loading, and the side support materials must be evenly added to the same compaction standards as the bedding materials so that the installed PE pipe is not disturbed.

Sidefill materials should be built up equally on both sides of the pipes in layers of 150mm, and compacted evenly to the AS/NZS 2566 design level. The sidefill materials must be carefully placed around the haunches of the pipes to ensure that the PE pipes are evenly supported.

Vibrating plate compactors must not be used until there is a 300mm layer of overlay soil over the crown of the PE pipe.

Detector tapes, or marker strips, should be laid on top of the overlay once a layer of 150mm soil has been compacted.

The overlay materials should be built up in compacted layers until the overlay material is to a level of a minimum of 150 mm above the top of the PE pipes. (See Figure 5.1). Large diameter (450 mm and above) PE pipes require the overlay materials to be carried to a cover of 300mm above the top of the PE pipes.

## Backfill

The remainder of the trench, or embankment fill may be made with the previously excavated native materials. These must be free from large rocks, vegetable matter, and contaminated materials, and all materials must have a maximum particle size less than 75 mm.

Where PE pipelines are installed in areas with high external loads, then the backfill materials must be of the same standard as the bedding and overlay materials.

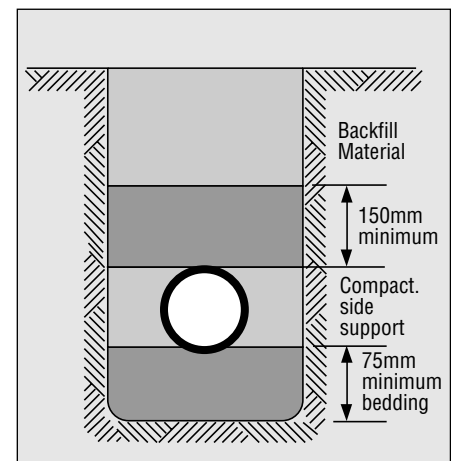
## Compaction Standards

It is essential that the AS/NZS 2566 compaction levels are attained, as PE pipes behave as flexible structures. Large diameter PE pipe installations may require the compaction at each stage of the installation to be confirmed by test.

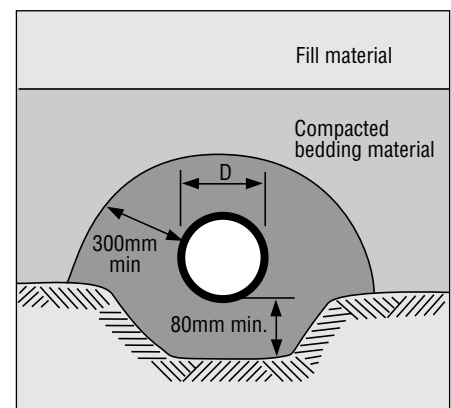
Where high external loads are encountered, or where it is not possible to attain the required level of compaction in the sidefill materials, a mixture of sand/cement in the ratio of 14:1 may be used in the sidefill zones.

The selection of compaction standard used in the sidefill materials needs to be taken from AS/NZS 2566 for the sidefill materials available on the particular site.

**Figure 5.1**  
**Trench Installations**



**Figure 5.2**  
**Embankment Installations**





## Relining & Sliplining

Vinidex PE pipes have the chemical resistance properties and longitudinal flexibility to provide an ideal solution for relining existing corroded or damaged pipelines in water supply, sewers, and drain applications.

Existing pipelines used to transport aggressive and dangerous fluids may be restored by relining techniques, and cost effective solutions are provided by eliminating the need for open cut trenches in urban and heavily built up areas. Installations can be planned around off peak traffic periods to minimise disruption and reduce installation times.

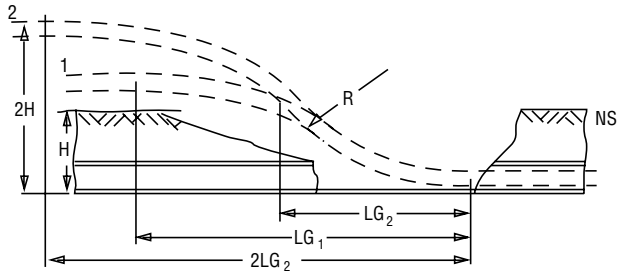
Existing pipelines can be renovated by inserting Vinidex PE pipes into the old pipes. Insertion pipes can be pulled into position by mechanical winches.

Although insertion of the PE pipes will reduce the internal diameter of the pipeline, the effective flow capacity of the renovated line may in fact be greater than the existing installation due to the improved pipe wall friction factors of PE as compared to the existing pipe with heavily corroded or damaged internal surfaces. Inspection of the existing line should be performed by CCTV to provide data as to the actual likely flow friction factors.

Relining with PE pipes provides a structural element that is capable of withstanding either internal pressure or external loading without relying on the residual strength of the original degraded pipe elements.



**Figure 5.3 PE Sliplining Trench Opening**



The PE pipes require short length inlet and exit trenches to accommodate the PE pipe radius to lead into the existing pipeline, and the winch assembly used to pull the PE liner along the pipeline. The minimum bending radius of the PE liner can be calculated as described under Pipeline Curvature in this section of the manual.

The dimensions (Refer to Figure 5.3) of excavations required for slip lining are:

1. Where the PE insert pipe is on the natural surface level

$$LG_1 = \sqrt{H(4R - H)}$$

2. Where the PE insert pipe is at a height H above the natural surface level

$$LG_2 = \sqrt{H(2R - H)}$$

where

H = depth to invert of existing pipeline

R = radius of liner pipe

### Grouting

Grouting of the gap between the outside diameter of the PE liner, and the inside of the existing pipe is necessary only when the original pipe has been damaged to the extent that there is no residual external load capacity, or where manhole connections cannot be sealed off to prevent groundwater infiltration.

Where grouting is applied, the pressure should not exceed 50 kPa, and depending on the PN rating of the PE liner pipe, external collapse calculations should be carried out. Where cement based grouts are used, the temperature rise in the PE liner due to the heat of hydration must be taken into account.

The PE liner pipes may be filled with water prior to grouting to increase the external pressure resistance, and to provide additional line weight to prevent the PE liner pipe floating during grouting, and losing the final grade line.



## Above Ground Installation

Vinidex PE pipes may be installed above ground for pressure and non pressure applications in both direct exposure and protected conditions.

Black PE pipes made to AS/NZS 4130 requirements may be used in direct sunlight exposure conditions without any additional protection. Where PE pipes of colours other than black are used in exposed conditions, then the pipes may need to be protected from sunlight.

Where PE pipes are installed in direct exposure conditions, then the increased PE material temperature due to exposure must be taken into account in establishing the operational pressure rating of the PE pipes. Localised temperature build up conditions such as proximity to steam lines, radiators, or exhaust stacks must be avoided unless the PE pipes are suitably protected.

Where lagging materials are used, these must be suitable for external exposure applications.

For Vinidex Geberit waste systems, the pipes are manufactured specifically for the application and reference should be made to Vinidex engineers for comprehensive installation details.

## Supports

Pipe hangers, or supports, should be located evenly along the length of the PE pipeline, and additionally at localised points with heavy items such as valves, and fittings.

The supports should provide a bearing surface of 120° under the base of the pipes. The PE pipes may need to be protected from damage at the supports. This protection may be provided by a membrane of PE, PVC or rubber.

Location and type of support must take into account provision for thermal movement, if required. If the supports are to resist thermal movement, an assessment of the stress induced in pipes, fittings and supports may need to be made.

## Support Spans

Support spans depend on the pipe material and dimensions, nature of flow medium, operating temperature, and arrangement of the pipes.

In calculating support spans, a maximum deflection of spans/500 between supports has been adopted as the basis.

The spans in Table 5.4 are based on the use of PE80B (MDPE), full of water, support over multiple spans, and operating at 20°C for 50 years.

For other service temperatures, the spans should be reduced as follows:

30°C	5%
40°C	9%
50°C	13%

For fluids with density between 1000 kg/m<sup>3</sup> and 1250 kg/m<sup>3</sup>, decrease spans by 4%.

For Vinidexair systems, the spans may be increased by up to 30%.

**Table 5.4 Support Spans (metres)**

DN	SDR (Standard Dimension Ratio)								
	41	33	26	21	17	13.6	11	9	7.4
16	0.55	0.55	0.55	0.55	0.55	0.55	0.55	0.55	0.55
20	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.65	0.65
25	0.65	0.65	0.65	0.65	0.65	0.70	0.70	0.75	0.75
32	0.70	0.70	0.70	0.70	0.75	0.80	0.85	0.90	0.90
40	0.80	0.80	0.80	0.80	0.90	0.90	1.00	1.00	1.10
50	0.85	0.85	0.90	0.95	1.00	1.10	1.15	1.20	1.25
63	0.95	1.00	1.05	1.10	1.20	1.25	1.30	1.40	1.45
75	1.00	1.10	1.20	1.25	1.35	1.40	1.50	1.55	1.60
90	1.15	1.25	1.35	1.40	1.50	1.60	1.65	1.75	1.80
110	1.35	1.40	1.55	1.60	1.70	1.80	1.90	2.00	2.10
125	1.45	1.55	1.65	1.75	1.85	2.00	2.10	2.20	2.30
140	1.55	1.65	1.80	1.90	2.00	2.10	2.25	2.35	2.45
160	1.70	1.80	1.95	2.10	2.20	2.30	2.45	2.55	2.65
180	1.85	1.95	2.10	2.25	2.35	2.50	2.65	2.80	2.90
200	1.95	2.10	2.25	2.40	2.55	2.70	2.85	3.00	3.10
225	2.15	2.30	2.45	2.60	2.75	2.90	3.05	3.20	3.35
250	2.30	2.45	2.60	2.75	2.95	3.10	3.30	3.45	3.60
280	2.45	2.65	2.80	3.00	3.20	3.35	3.55	3.70	3.90
315	2.65	2.85	3.05	3.25	3.45	3.65	3.85	4.05	4.20
355	2.90	3.10	3.30	3.50	3.75	3.95	4.15	4.35	4.55
400	3.10	3.35	3.55	3.80	4.05	4.25	4.50	4.70	4.90
450	3.40	3.60	3.85	4.10	4.35	4.60	4.85	5.10	5.35
500	3.60	3.85	4.15	4.40	4.70	4.95	5.20	5.50	
560	3.90	4.15	4.50	4.75	5.05	5.35			
630	4.20	4.50	4.85	5.15	5.45	5.80			
710	4.60	4.90	5.25	5.60	5.95	6.30			
800	4.95	5.30	5.70	6.05	6.45	6.85			
900	5.35	5.70	6.10	6.55	6.95				
1000	5.80	6.15	6.55	7.00	7.35				

### Expansion & Contraction

For above ground pipelines, expansion and contraction movements should be taken up by the pipeline where possible without expansion joints.

This may be achieved in lines laid directly on the natural surface by snaking the pipe during installation and allowing the pipe to move freely in service. Where the final joint connections are made in high ambient temperature, sufficient pipe length must be allowed to permit the pipe to cool, and hence contract, without pulling out of non end load bearing joints.

## Accommodation of Thermal Movement by Deflection Legs

Changes in length are caused by changes in operating temperatures. On installation of piping systems above ground, attention must be paid to compensate for axial movements.

In most cases, changes in direction in the run of piping may be used to absorb length change, given that appropriate deflection legs are provided. Otherwise, compensation loops or special fittings may need to be installed.

Table 5.5 lists minimum deflection leg lengths for given run length changes. See Figures 5.4 and 5.5.

For non-pressure applications, these values may be reduced by 30%, or for Vinidex Geberit systems, up to 60%. For specific data, reference should be made to Vinidex engineers.

The deflection leg is expressed by:

$$L_s = k \cdot \sqrt{\Delta L \cdot DN} \text{ [mm]}$$

where

$L_s$  = deflection leg (mm)

$\Delta L$  = change in length (mm)

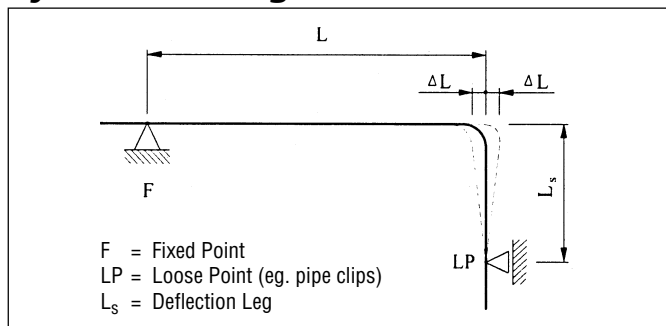
DN = pipe outside diameter (mm)

$k$  = material specific proportionality factor (average value for PE of 26)

**Table 5.5 Minimum Deflection Leg Lengths (m)**

DN	Change in Run length $\Delta L$ (mm)									
	50mm	100mm	150mm	200mm	250mm	300mm	350mm	40mm	450mm	
16	0.75	1.05	1.30	1.50	1.65	1.85	1.95	2.10	2.35	
20	0.85	1.15	1.45	1.65	1.85	2.05	2.20	2.35	2.60	
25	0.95	1.30	1.60	1.85	2.10	2.25	2.45	2.60	2.90	
32	1.05	1.50	1.85	2.10	2.35	2.55	2.80	2.95	3.30	
40	1.15	1.65	2.05	2.35	2.60	2.85	3.10	3.30	3.70	
50	1.30	1.85	2.25	2.60	2.90	3.20	3.50	3.70	4.15	
63	1.50	2.10	2.55	2.95	3.30	3.60	3.85	4.20	4.65	
75	1.60	2.25	2.80	3.20	3.60	3.90	4.25	4.50	5.05	
90	1.80	2.50	3.05	3.50	3.90	4.30	4.65	4.95	5.55	
110	1.95	2.75	3.40	3.85	4.35	4.75	5.15	5.50	6.15	
125	2.10	2.90	3.55	4.15	4.60	5.05	5.50	5.85	6.55	
140	2.20	3.10	3.80	4.40	4.90	5.35	5.80	6.20	6.90	
160	2.35	3.30	4.05	4.70	5.20	5.75	6.20	6.60	7.40	
180	2.50	3.50	4.30	4.95	5.55	6.10	6.55	7.00	7.80	
200	2.60	3.70	4.50	5.20	5.85	6.35	6.90	7.40	8.25	
225	2.80	3.90	4.85	5.55	6.20	6.80	7.35	7.85	8.80	
250	2.90	4.15	5.05	5.85	6.55	7.20	7.75	8.25	9.20	
280	3.10	4.35	5.35	6.20	6.90	7.55	8.20	8.70	9.80	
315	3.30	4.65	5.70	6.55	7.35	8.05	8.70	9.25	10.35	
355	3.50	4.90	6.05	6.95	7.80	8.55	9.20	9.85	11.00	
400	3.70	5.20	6.40	7.40	8.25	9.05	9.80	10.45	11.70	
450	3.90	5.55	6.80	7.85	8.80	9.60	10.40	11.10	12.40	
500	4.15	5.85	7.20	8.25	9.25	10.15	10.90	11.70	-	
560	4.40	6.20	7.55	8.75	9.80	10.70	-	-	-	
630	4.65	6.55	8.05	9.25	10.40	11.35	-	-	-	
710	4.90	6.95	8.55	9.80	11.00	12.05	-	-	-	
800	5.25	7.40	9.10	10.50	11.75	12.80	-	-	-	
900	5.60	7.90	9.65	11.10	12.50	-	-	-	-	
1000	5.85	8.30	10.15	11.70	13.10	-	-	-	-	

**Figure 5.4 Absorption of change in length by deflection leg**



**Figure 5.5 Absorption of change in length by a compensation elbow**

